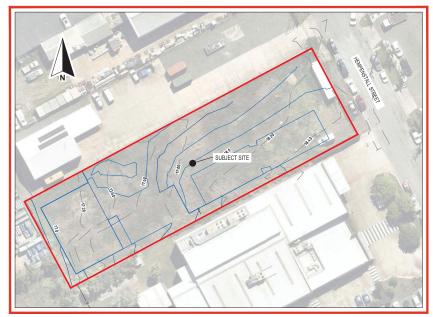
NEW WAREHOUSE AND OFFICE

11-13 HEMPENSTALL STREET, KAWANA QLD 4701

NOVUS LOGISTICS

D24.054 CIVIL DESIGN



LOCALITY PLAN (Not To Scale)



ACN 121 309 171 47 Normanby Street Yeppoon, Queensland 4703

 Phone:
 07 49112553

 Fax:
 07 49383660

 Email:
 admin@dileigh.com.au

ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with

Development Permit No.: D/103-2024 Dated: 10 December 2024

DESIGN DRAWING LIST INDEX								
SHEET NUMBER	SHEET TITLE							
D24.054 C-00	TITLE PAGE							
D24.054 C-01	EXISTING FEATURES & SERVICES							
CIVIL: ACCESS AND PARKING								
D24.054 C-02	PROPOSED LAYOUT							
D24.054 C-03	SETOUT PLAN							
D24.054 C-04	SITE LONG SECTIONS 1 & 2							
D24.054 C-05	SITE CROSS SECTIONS CONTROL LINE 1 SH 1 OF 2							
D24.054 C-06	SITE CROSS SECTIONS CONTROL LINE 1 SH 2 OF 2							
D24.054 C-07	EARTHWORKS PLAN							
D24.054 C-08	ACCESS AND PARKING							
D24.054 C-09	CONCRETE PAVEMENT JOINT LAYOUT PLAN							
D24.054 C-10	MASONRY DETAILS							
CIV	IL: STORMWATER							
D24.054 C-11	STORMWATER LAYOUT PLAN							
D24.054 C-12	STORMWATER LONGITUDINAL SECTIONS							
CIVIL: EROSIO	ON AND SEDIMENT CONTROL							
D24.054 C-13	EROSION SEDIMENT CONTROL PLAN							
D24.054 C-14	EROSION CONTROL NOTES AND RETAINING WALL DETAIL							

OPERATIONAL WORKS ISSUE

FOR CONSTRUCTION ONLY WITH COUNCIL APPROVAL

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_____ EXISTING EASEMENT

61.00 EXISTING SURFACE CONTOURS

EXISTING LEVELS AND SERVICES

- THE CONTRACTOR SHALL VERIFY THE LOCATIONS AND LEVELS OF ALL EXISTING SERVICES WITH THE RELEVANT AUTHORITIES INCLUDING "DIAL BEFORE YOU DIG" PRIOR TO COMMENCING CONSTRUCTION.
- ANY COSTS ASSOCIATED WITH REPAIRING DAMAGE TO EXISTING SERVICES SHALL BE PAID FOR BY THE CONTRACTOR.
 THE CONTRACTOR SHALL VERIFY THAT THE EXISTING LEVELS ARE AS PER
- THIS DESIGN WHERE CONNECTIONS TO EXISTING INFRASTRUCTURE ARE REQUIRED. ANY DIFFERENCES TO BE NOTIFIED TO THE ENGINEER PRIOR TO ORDERING MATEFIALS OR COMMENCING WORKS. 4. PRIOR TO COMMENCING WORKS THE CONTRACTOR SHALL VERIFY THAT
- 4, PADE TO COMMENDING WORKS THE CONTRACTOR STALL VERTITIAT THERE ARE NO CLASHES BETWEEN ANY CROSSING SERVICE OR PIPELINE. ANY CLASHES TO BE NOTIFIED TO THE ENGINEER PRIOR TO WORKS COMMENCING.
- 5. PRIOR TO COMMENCING WORKS THE CONTRACTOR SHALL VERIFY LOCATION AND DETALS OF ALL EXISTING SERVICE CONNECTIONS TO NEW ALLOTMENTS PREVIOUSLY INSTALLED

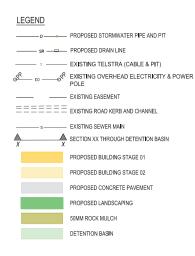


ROCKHAMPTON REGIONAL COUNCIL APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with **Development Permit No.: D/103-2024**

Dated: 10 December 2024

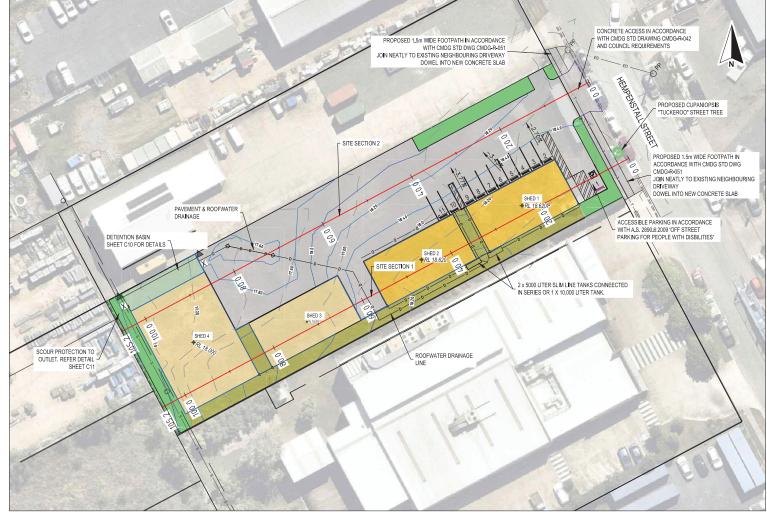
DATUM: HORIZ, GDA 94 VERT, AHD		REV REVISION DESCRIPTION	DATE		DRAFTED	CWR	NOVUELOCIETICS	DWG No.
		A FOR DISCUSSION	03/05/2024	ACN 121 309 171	DESIGNED	SJG	NOVUS LOGISTICS	D24.054-C01
	OPERATIONAL WORKS ISSUE	C MINOR AMENDMENTSMINOR AMENDMENTS	27/06/2024	47 Normanhy Street	CHECKED	ACD	NEW WAREHOUSE AND OFFICE	D24.004 001
0 2.5 5 7.5 10 HORIZONTAL 1:500					ADDOOLED	GJBROWN	11-13 HEMPENSTALL STREET, KAWANA QLD 4701	CIVIL
HURIZUWIAL	FOR CONSTRUCTION ONLY WITH COUNCIL APPROVAL				RPEQ 7682		OPERATIONAL WORKS	REVISION
EUL AO	FOR CONSTRUCTION ONET WITH COUNCIL AFFROVAL		+	CIVIL / STRUCTURAL DESIGN & PROJECT MANAGEMENT Email: admin@dleigh.com.au		Charles	EXISTING FEATURES AND SERVICES	C
SCALES m. FULL A3			+		11.09.2024	12/1	ENDTING FEATURED AND DERVICED	0





1. 0.5% MINIMUM GRADE, 1% DESIRABLE ON ALL CONCRETE SURFACES. 2. REFER CMDG STANDARD DRAWING CMDG-R-042 FOR CROSSOVER DETAIL 3. REFER DRAWING D24.054 - C04 TO C05 FOR SITE PROFILES & SECTIONS 4. REFER DRAWING D24.054 - C07 FOR EARTHWORKS PLAN 5. REFER DRAWING D24.054 - C08 FOR ACCESS AND PARKING PLAN 6. REFER DRAWING D24.054 - C11 FOR STORM WATER LAYOUT PLAN 7. REFER DRAWING D24.054 - C13&14 FOR EROSION AND SEDIMENT CONTROL PLAN & NOTES

ROCKHAMPTON REGIONAL COUNCIL APPROVED PLANS



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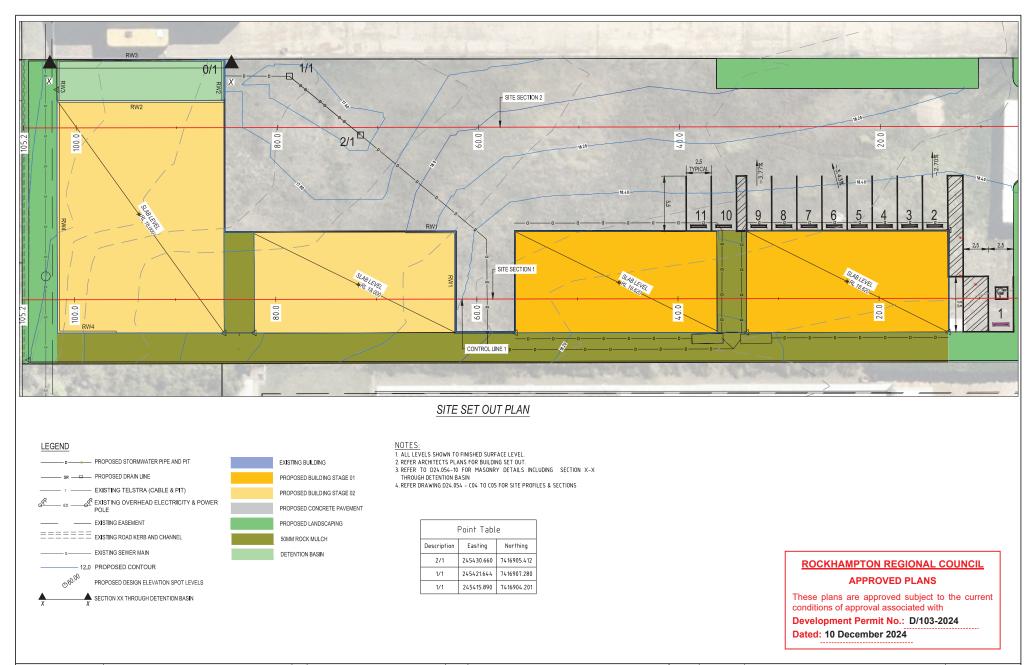
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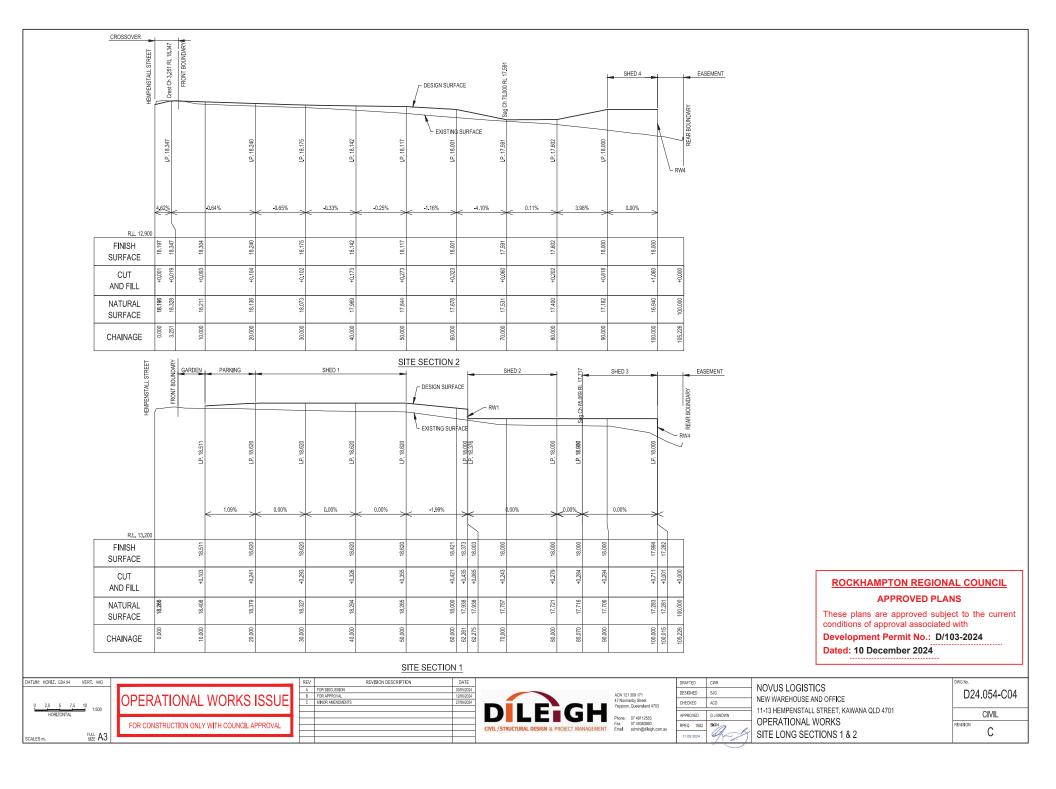
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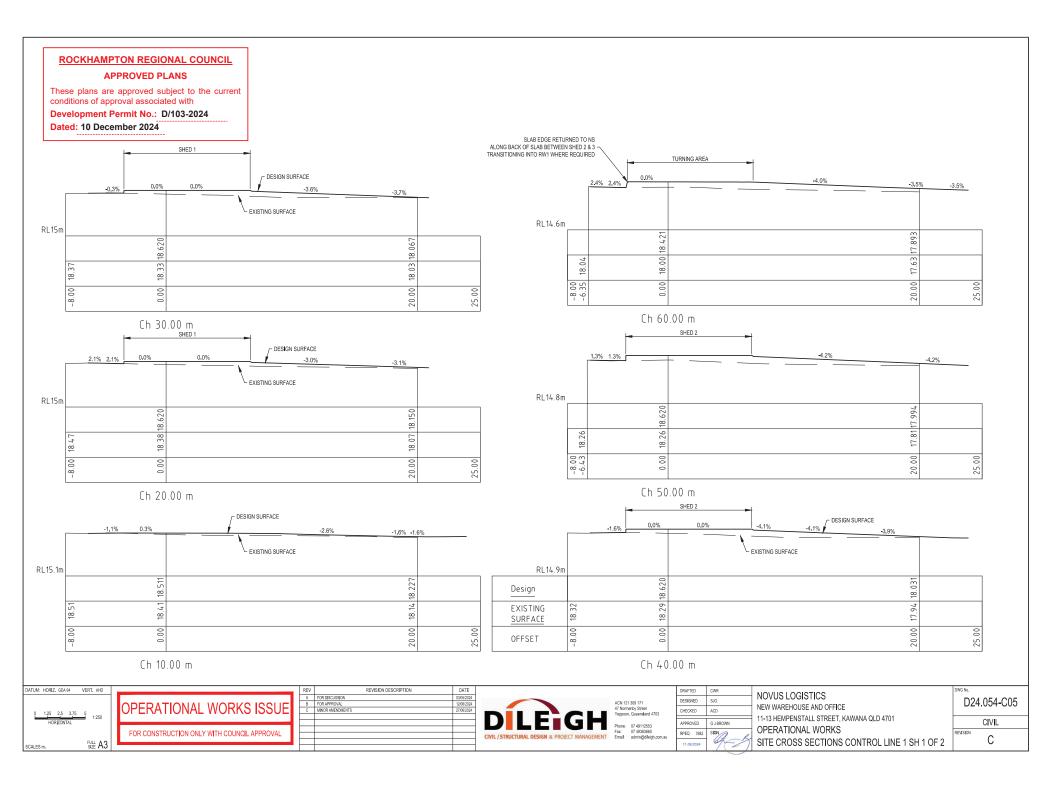
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DATUM: HOR	Z. GDA 94 VERT. AHD		REV REVISION DESCRIPTION	DATE 03/05/2024			CWR	NOVUS LOGISTICS	
		OPERATIONAL WORKS ISSUE		12/06/2024	ACN 121 309 171 47 Normanby Street	DESIGNED CHECKED	SJG	NEW WAREHOUSE AND OFFICE	D24.054-C02
0 2.5	5 7.5 10 VERTICAL 1:500		D PIPE LAYOUT AMENDED E STREET TREE AND FOOTPATH ADDED	10.09.2024 18/10/2024	A Normarby Street Vepport, Cardinal 4703 Prote: 07 4513558 Fax: 07 4503580	APPROVED	GJBROWN	11-13 HEMPENSTALL STREET, KAWANA QLD 4701	CIVIL
		FOR CONSTRUCTION ONLY WITH COUNCIL APPROVAL			Fia: 07 49333660 CIVIL / STRUCTURAL DESIGN & PROJECT MANAGEMENT Eas: 07 49333660 Email: admin@deleiph.com.au	RPEQ 7682	SIGN	OPERATIONAL WORKS	REVISION
	t ID: 40928465					24.10.2024	42	PROPOSED LAYOUT	E Š

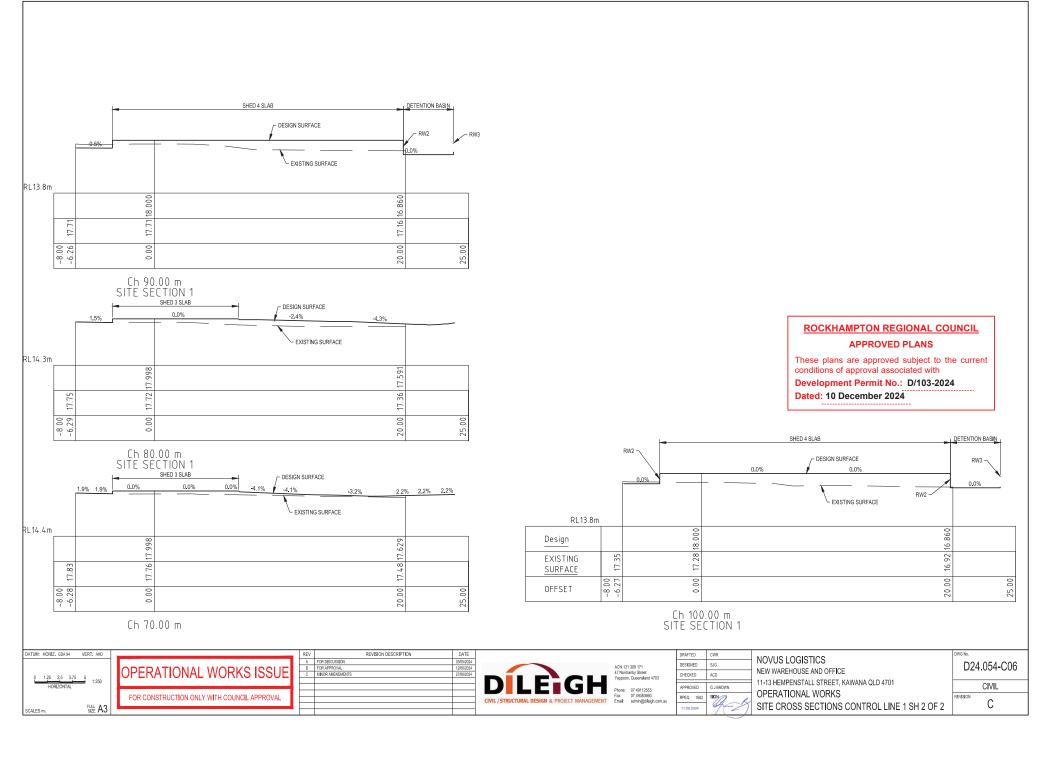
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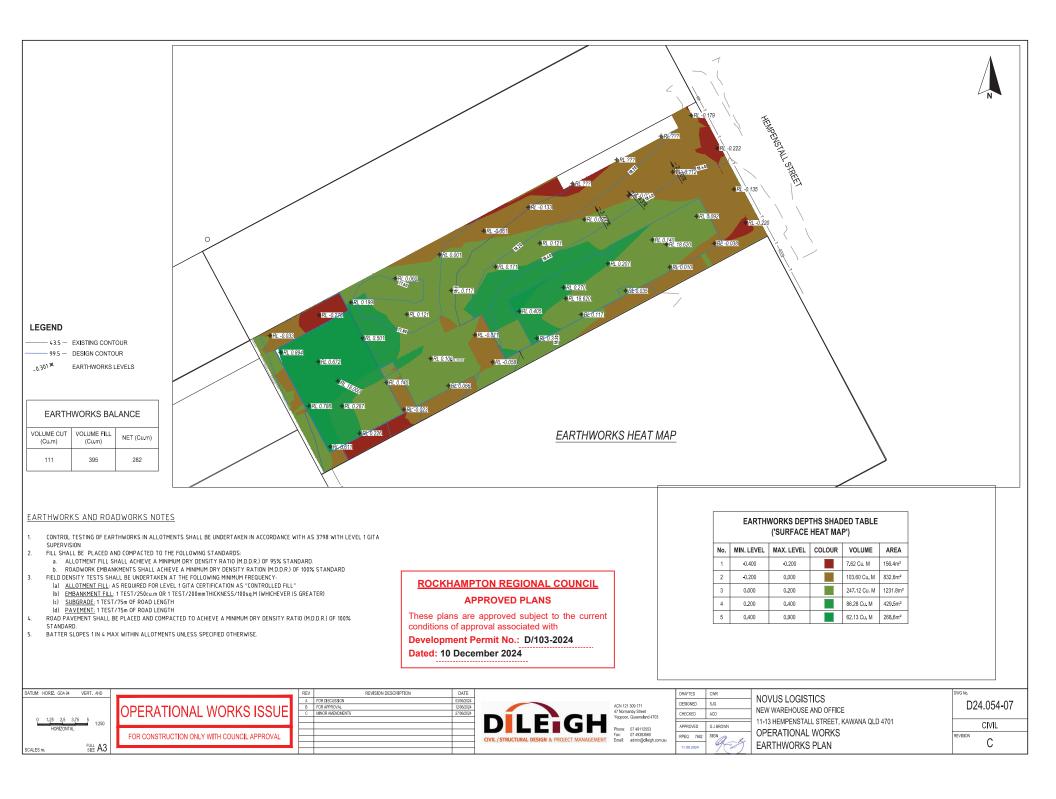


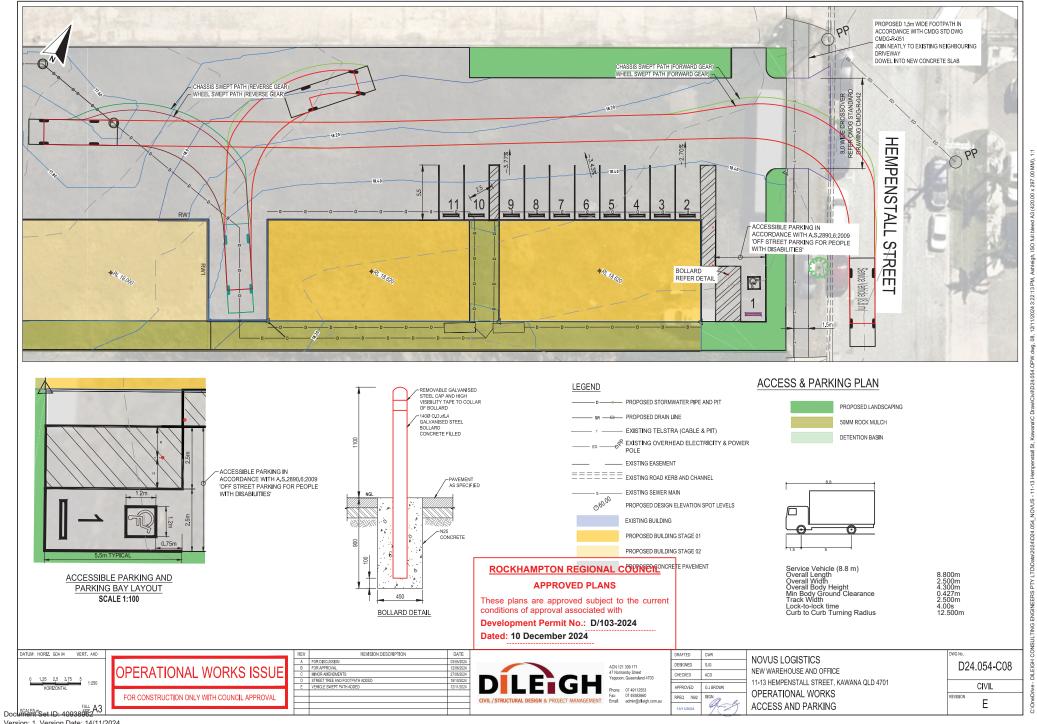
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		A	FOR DISCUSSION	03/05/2024	ACN 121 309 171	DESIGNED	SJG	NOVUS LOGISTICS	D24.054-C03
	OPERATIONAL WORKS ISSUE	C	FOR APPROVAL MINOR AMENDMENTS	27/06/2024	How is 1 June 1		400	NEW WAREHOUSE AND OFFICE	D24.034-003
0 1.25 2.5 3.75 5 1.250			PIPE LAYOUT AMENDED	10,09,2024	Yeppoon, Queensiand 4/03	UNCONCO	NOD	11-13 HEMPENSTALL STREET, KAWANA QLD 4701	0.11 //
HORIZONTAL					Phone: 07 49112553	APPROVED	G J BROWN		CIVIL
	FOR CONSTRUCTION ONLY WITH COUNCIL APPROVAL	-			CIVIL / STRUCTURAL DESIGN & PROJECT MANAGEMENT Fax: 07 49383660 Emai: 07 49383660 admin@dleigh.com.au	RPEQ 7682	SIGN	OPERATIONAL WORKS	REVISION
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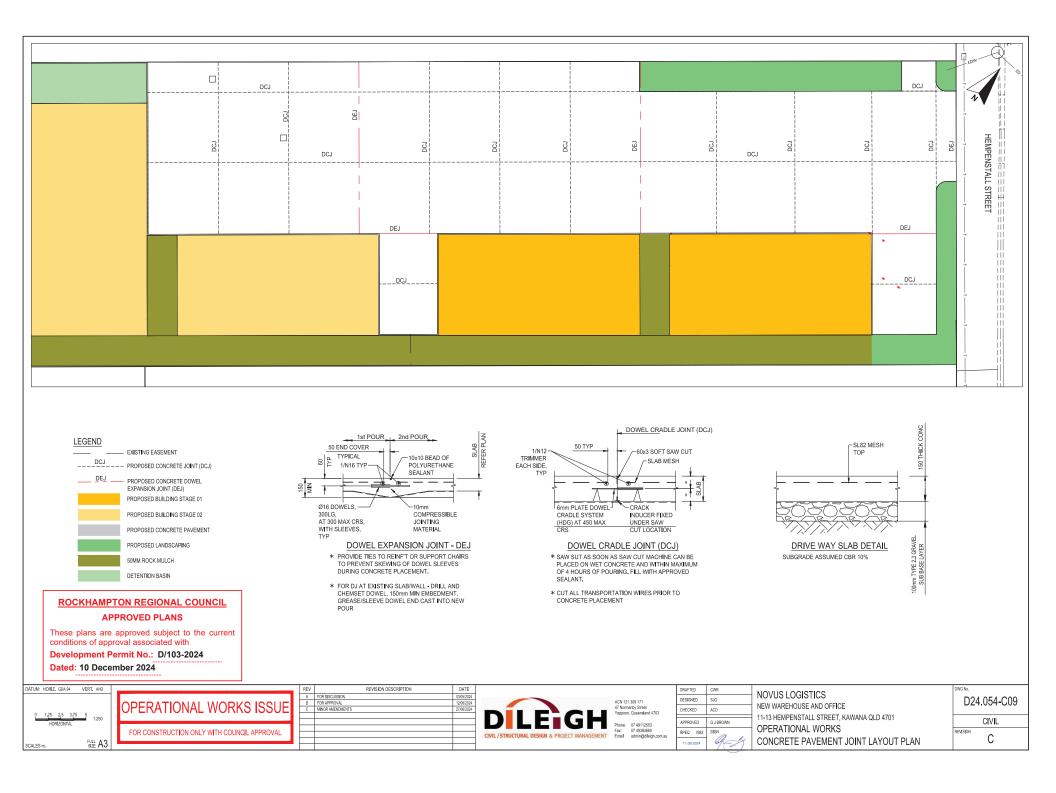


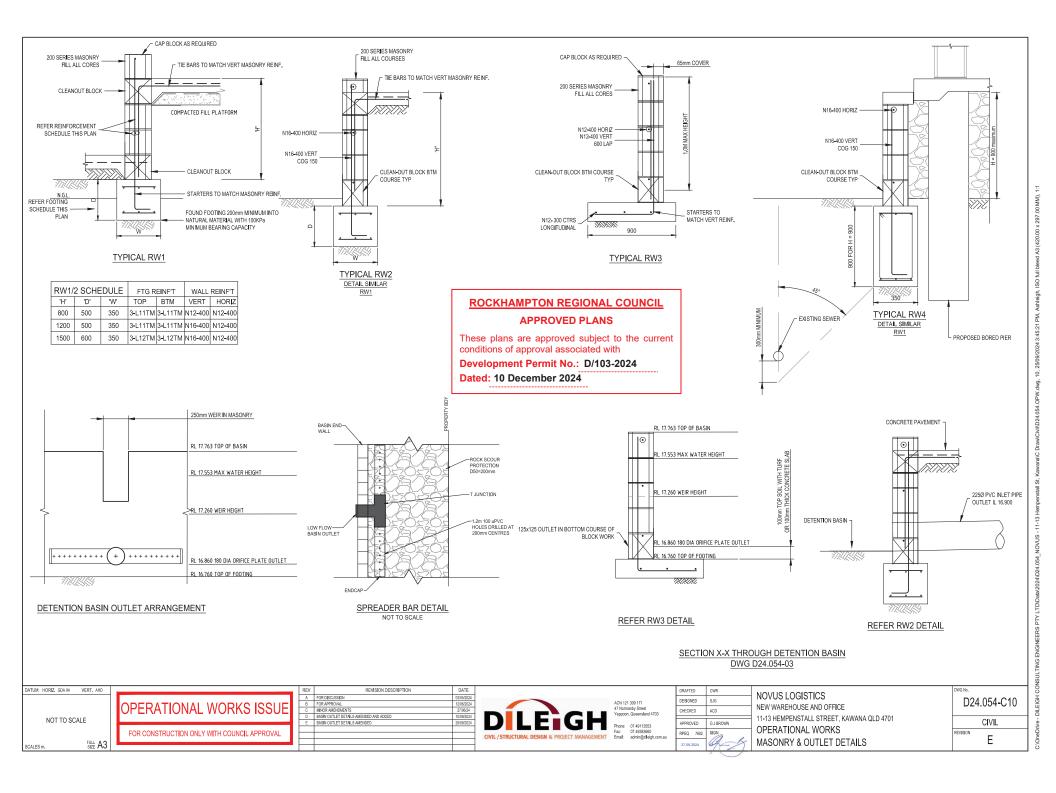


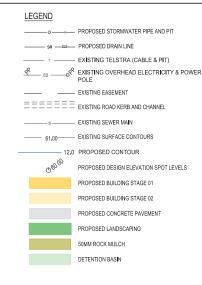




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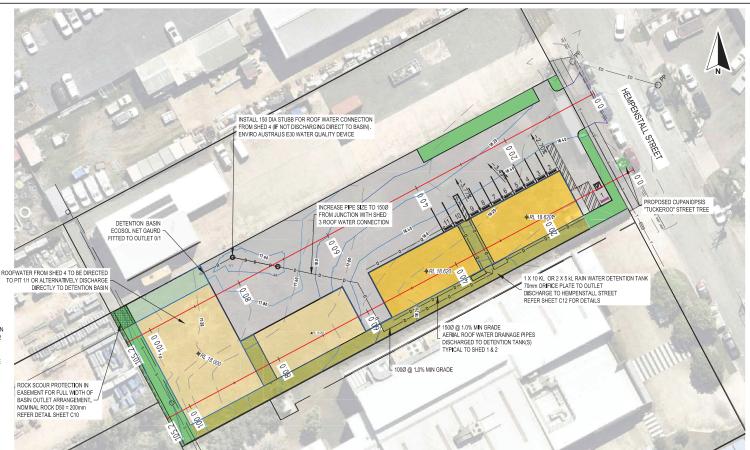






DRAINAGE LONGITUDINAL SECTION NOTES:

- 1. PIPED NETWORK MODELED AND LONGITUDINAL SECTION GENERATED BY ARD PIPES.
- PIPE NETWORK FOR GROUND INLET PITS MODELED FOR Q20 MINOR EVENT IN ACCORDANCE WITH CMDG STORMWATER DESIGN GUIDELINE TABLE 0.5.04.2 FOR COMMERCIAL DEVELOPMENT.
- PIPE NETWORK FOR ROOFWATER PIPES MODELED FOR Q20 EVENT IN ACCORDANCE WITH AS 3500.3.2 1998 STORMWATER DRAINAGE ACCEPTABLE SOLUTIONS.
- MAJOR AND MINOR RAINFALL INTENSITIES GENERATED USING BUREAU OF METEOROLOGY 2016 RAINFALL IFD DATA SYSTEM.
 REFER TO DRAWING 024,054-C12 FOR STORM WATER LONG SECTIONS AND
- 5. REFER TO DRAWING 024.054-C12 FOR STORM WATER LONG SECTIONS AND DETAILS
- 5. REFER TO DRAWING D24.054-C10 FOR DETENTION BASIN MASONRY DETAILS



STORM WATER LAYOUT

ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with **Development Permit No.: D/103-2024**

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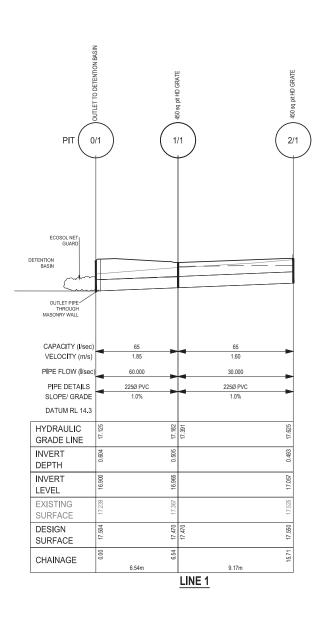
Dated: 10 December 2024

DATUM: HORIZ. GDA 94 VERT. AHD		REV	REVISION DESCRIPTION	DATE		DRAFTED	CWR		DWG No.
		A	FOR DISCUSSION FOR APPROVAL	03/05/2024	ACN 121 309 171	DESIGNED	SJG	NOVUS LOGISTICS	D24.054-C11
0 1.25 2.5 3.75 5	OPERATIONAL WORKS ISSUE	C	MINOR AMENDMENTS	27/06/2024	47 Normanby Street Yeopoon, Queensland 4703	CHECKED	ACD	NEW WAREHOUSE AND OFFICE	D24.004-011
HORIZONTAL 1:250		D	PIPE LAYOUT AMENDED TANK ORFICE AMENDED	10/09/2024	Yeppoon, Queensiand 4703			11-13 HEMPENSTALL STREET, KAWANA QLD 4701	CIVIL
Tongeowithe		F	STREET TREE AND FOOTPATH ADDED	18/10/2024	Phone: 07 49112553 Fax: 07 49383660		G J BROWN	OPERATIONAL WORKS	
	FOR CONSTRUCTION ONLY WITH COUNCIL APPROVAL				CIVIL / STRUCTURAL DESIGN & PROJECT MANAGEMENT Email: admin@dleigh.com.au	RPEQ 768	SIGN L		
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DRAINAGE LONGITUDINAL SECTION NOTES:

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- 1. PIPED NETWORK MODELED AND LONGITUDINAL SECTION GENERATED BY ARD PIPES.
- 2. PIPE NETWORK FOR GROUND INLET PITS MODELED FOR Q20 MINOR EVENT IN ACCORDANCE WITH CMDG STORMWATER DESIGN GUIDELINE TABLE 0.5.04.2 FOR COMMERCIAL DEVELOPMENT.
- PIPE NETWORK FOR ROOFWATER PIPES MODELED FOR Q20 EVENT IN 3. ACCORDANCE WITH AS 3500.3.2 1998 STORMWATER DRAINAGE ACCEPTABLE SOLUTIONS.
- MAJOR AND MINOR RAINFALL INTENSITIES GENERATED USING BUREAU OF METEOROLOGY 2016 RAINFALL IFD DATA SYSTEM. 4.
- REFER TP DRAWING D24.054-C09 FOR STORM WATER LAYOUT PLAN 5
- **ROCKHAMPTON REGIONAL COUNCIL** APPROVED PLANS -ROOF SHEETING These plans are approved subject to the current -ROOF GUTTER conditions of approval associated with Development Permit No.: D/103-2024 Dated: 10 December 2024 DOWN PIPE 1500mmTANK OVERFLOW OUTLET TANK TOP WATER LEVEL TOP WATER LEVEL -150ØmmTANK OVERFLOW OUTLET DETENTION STORAGE DETENTION STORAGE -100ØmmTANK OVERFLOW DOWNPIPE 1000mm LOW ELOW BOTTOM WATER OUTLET WITH 70Ø ORIFICE PLATE BOTTOM WATER LEVEL

L₄₅₀ SQ PIT

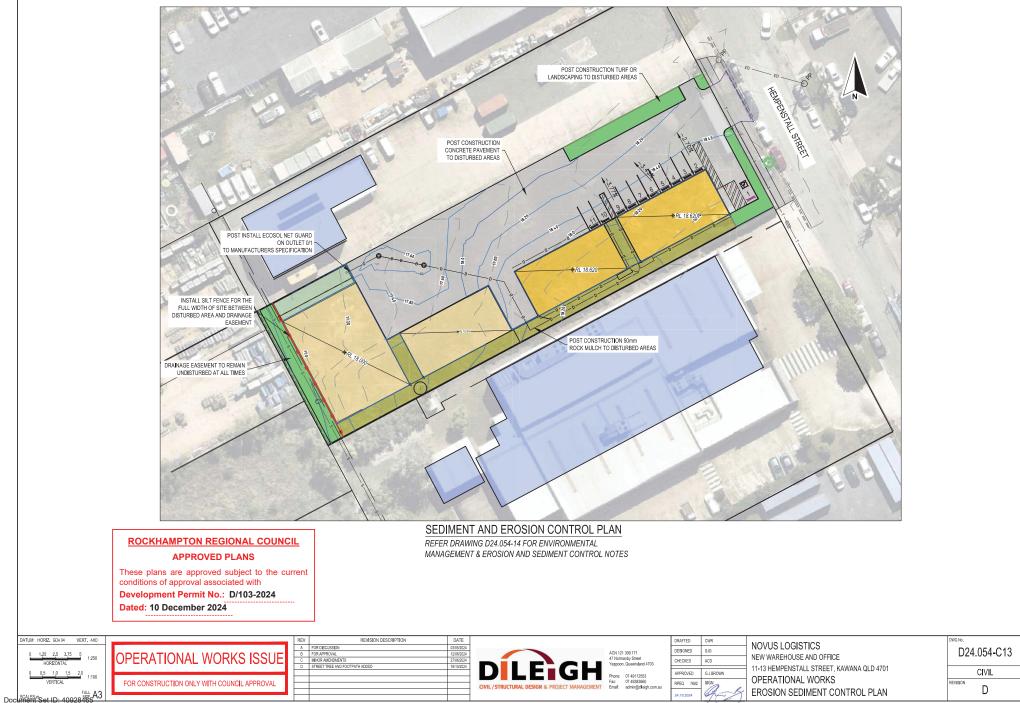
1000 PVC PIPE LINK BETWEEN 5kL TANKS ORIFICE OUTLET ON ONE TANK ONLY NOT REQUIRED IF USING 1 X 10kL TANK

ROOF WATER DETENTION TANK ARRANGEMENT

1000mm PVC PIPE OUTLET. REFER LAYOUT PLAN FOR LOCATION AND LENGTH

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ſ	DATUM: HORIZ GDA 94 VERT. AHD		REV	REVISION DESCRIPTION	DATE		DRAFTED	CWR		DWG No.
			A	FOR DISCUSSION NOTATION AMENDED	27/05/2024	ACN 121 200 171	DESIGNED	SJG	NOVUS LOGISTICS	D24.054-C12
	0 1.25 2.5 3.75 5 1:250	PRELIMINARY ISSUE	C	TANK ORIFICE AMENDED	10/09/2024 25/09/2024	ACN 121 309 171 47 Normanby Street	CHECKED	ACD	NEW WAREHOUSE AND OFFICE	D24.004-012
	HORIZONTAL					Yeppoon, Queensland 4703			11-13 HEMPENSTALL STREET, KAWANA QLD 4701	011/11
	0 0.5 1.0 1.5 2.0		-			Phone: 07 49112553	APPROVED	G J BROWN	OPERATIONAL WORKS	CIVIL
	VERTICAL	FOR DISCUSSION	-			CIVIL / STRUCTURAL DESIGN & PROJECT MANAGEMENT Email: admin@dleigh.com.au	RPEQ 7682	SIGN		REVISION
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Version: 1, Version Date: 25/10/2024

ENVIRONMENTAL MANAGEMENT NOTES:

- PRIOR TO THE COMMENCEMENT OF EARTHWORKS. TOPSOIL SHALL BE STRIPPED AND STOCKPILED FROM SELECT AREAS ONLY FOR RE-SPREADING OVER DISTURBED AREAS PRIOR TO REVEGETATION AND LANDSCAPING
- PRIOR TO THE COMMENCEMENT OF ANY EARTHWORKS ALL SEDIMENT CONTROL DEVICES WILL BE ERECTED WHERE SHOWN ON THE DRAWINGS OR OTHERWISE DIRECTED BY THE ENGINEER.
- ALL DISTURBED AREAS ON-SITE AND IN ROAD RESERVE WILL BE RE-TOPSOILED, TURFED OR LANDSCAPED
- ALL SOIL CONSERVATION AND ENVIRONMENTAL PROTECTION MEASURES SHALL BE MONITORED BY 4. THE CONTRACTOR AT REGULAR INTERVALS DURING CONSTRUCTION. SEDIMENT CONTROL DEVICES WILL BE MONITORED AFTER RAIN EVENTS AND MADE GOOD WHERE NECESSARY. THIS WILL ALSO BE CARRIED OUT DURING THE DEFECTS LIABILITY PERIOD
- SILT FENCES SHALL BE INSTALLED ON THE LOW SIDE OF ALL STOCKPILES WHERE REQUIRED
- SILT FENCES SHALL REMAIN ON SITE UNTIL ALL CONSTRUCTION ACTIVITIES ARE COMPLETE AND THERE IS 90% VEGETATION COVERAGE OF PROPOSED LANDSCAPED AREAS

NOISE MANAGEMENT:

- WORKING HOURS WORKING HOURS FOR THE SITE ARE TO BE 6.30am TO 6.30pm MONDAY TO SATURDAY, NO WORK TO BE UNDERTAKEN OUTSIDE OF TIMES SPECIFIED UNDER ANY CIRCUMSTANCES
- NOISE MINIMISATION METHODS NOISE WILL BE MINIMISED USING THE FOLLOWING METHODS:-2.
- RESTRICTED WORKING HOURS AS DETAILED ABOVE 2.1.
- 2.2. NOISE GENERATING MACHINERY TO OPERATED ONLY WHEN NECESSARY TO UNDERTAKE WORKS - VEHICLES AND MACHINERY ARE NOT TO BE LEFT 'IDLING' WHEN NOT IN USE. 2.3. NOISE SHIELDING ON PLANT TO BE INSPECTED PRIOR TO COMMENCEMENT OF WORKS AND
- MADE GOOD WHERE FOUND TO BE FAULTY. VEHICLES AND MACHINERY TO BE REGULARLY MAINTAINED TO REDUCE ENGINE NOISE 2.4.
- THROUGH INFREQUENT MAINTENANCE.

DUST MANAGEMENT:

- MINIMISING DUST GENERATION THE FOLLOWING WORK PRACTICES WILL BE USED TO MINIMISE 1. DUST GENERATION:
- 1.1. WIND CONDITIONS ON SITE ARE TO BE MONITORED AND SITE WORKS STOPPED IF WIND STRENGTH IS SUCH THAT EFFORTS TO MINIMISE AND/OR SUPPRESS DUST ARE INEFFECTIVE. 4.
- 1.2. SOIL STABILISATION OF BATTERS (THROUGH TOPSOILING AND REVEGETATION) TO BE UNDERTAKEN IMMEDIATELY AFTER FINAL TRIM TO MINIMISE EXPOSURE OF BARE EARTH.
- 1.3. STOCKPILES INTENDING TO BE LEFT IN PLACE FOR 28 DAYS OR GREATER SHALL BE GRASS
- SEEDED 2. DUST SUPPRESSION -
- 2.1. WET DOWN DUST GENERATING SURFACES DAILY PRIOR TO COMMENCEMENT OF WORK USING WATER TRUCKS, SPRINKLERS AND HOSE WATERING BY HAND.
- 22 ADDITIONAL WETTING DOWN OF SITE AREAS IS TO BE UNDERTAKEN AS NEEDED DURING THE COURSE OF THE DAY WHERE WORK AREAS HAVE DRIED AND ARE GENERATING DUST.

WEED MANAGEMENT:

- MOVEMENT OF SOIL EXISTING TOP SOIL IS TO BE STOCKPILED AND RE-USED ON SITE AFTER SITE WORKS ARE COMPLETE, ANY ADDITIONAL TOP SOIL REQUIRED IS TO BE FREE OF PLANT SEEDS PRIOR TO SPREADING ON SITE
- FILL MATERIAL FILL MATERIAL TO BE IMPORTED ON SITE IS TO BE 'CLEAN FILL' AND FREE FROM ANY 2. ORGANIC MATTER OR MATERIALS

EMERGENCY VEHICLE ACCESS:

MAINTAIN CLEAR ACCESS TO SITE FOR EMERGENCY VEHICLES AT ALL TIMES

WASTE MANAGEMENT

- ALL LITTER AND WASTE TO BE CONTAINED ON SITE IN CONTAINERS PROVIDED FOR THAT PURPOSE.
- ALL WASTE TO BE FURTHER DISPOSED OFF SITE IN A RESPONSIBLE MANNER. 2
- 3. WHERE POSSIBLE MINIMISE WASTE THROUGH WASTE MINIMIZATION AND RE-USE.

EROSION AND SEDIMENT MANAGEMENT:

- DRAINAGE MANAGEMENT WHERE POSSIBLE, RAINWATER DISCHARGE FROM UPSTREAM PROPERTIES IS TO BE DIRECTED AWAY FROM WORKS THROUGH TEMPORARY BUNDING.
- 1. SOIL STABILISATION -
- EXPOSED FARTH SHALL BE TOPSOILED. VEGETATED, AND LANDSCAPED AS SOON AS 1.1. POSSIBLE AFTER TRIMMING
- RE-VEGETATED AND LANDSCAPED AREAS SHALL BE REGULARLY WATERED TO ASSIST 12 ESTABLISHMENT OF COVER
- ALL BANKS AND BATTERS ARE TO BE REGULARLY INSPECTED TO IDENTIFY AREAS OF 13 EROSION AND RESHAPED TO PREVENT FURTHER EROSION IF NECESSARY - RECTIFICATION WORKS ARE TO BE RE-VEGETATED IMMEDIATELY.

STOCKPILE PROTECTION -2

- 2.1. STOCKPILES ARE TO BE SITUATED SUCH THAT THEY ARE NOT IN ANY STORMWATER FLOW PATHS
- 2.2. SILT FENCING IS TO BE INSTALLED TO DOWNSTREAM SIDE OF STOCKPILE AREAS PRIOR TO THEIR USE.
- STOCKPILES INTENDING TO BE LEFT IN PLACE FOR 28 DAYS OR GREATER SHALL BE GRASS 2.3 SEEDED.
- STOCKPILES TO HAVE A MAXIMUM SLOPE OF 2H:1V. 2.4.
- 3. SEDIMENT TRAPS -3.1. SILT FENCING & SEDIMENT TRAPS TO BE INSTALLED AT AREAS OF SITE DISCHARGE AS
- SHOWN ON PLAN.
- SILT FENCING TO BE INSTALLED TO DOWNSTREAM SIDE OF STOCKPILE AREAS, STRIPPED 3.2. AREAS, AND ANY OTHER AREAS OF BARE EARTH WHERE SILT LADEN RUNOFF CAN BE GENERATED.
- 3.3. SEDIMENT FENCING TO BE INSTALLED IN ACCORDANCE WITH SEDIMENT FENCE DETAILS ON THIS SHEET.
- SEDIMENT FENCE LAYOUT SHALL CONFORM TO "TYPICAL LAYOUT ACROSS GRADE" AS 3.4. DETAILED ON STANDARD DRAWING CMDG-D-050.
- SILT FENCES AND SEDIMENT TRAPS SHALL REMAIN ON SITE UNTIL ALL CONSTRUCTION 3.5 ACTIVITIES ARE COMPLETE AND THERE IS 90% VEGETATION COVERAGE OF PROPOSED. LANDSCAPED AREAS
- VEHICLE AND ROAD MANAGEMENT:
- 4.1. VEHICLES AND PLANT ARE TO ONLY ACCESS THE SITE FROM XXXXXX SITE ACCESS TO BE OVER A SHAKER ACCESS PAD OR RUMBLE GRID IN ACCORDANCE WITH STANDARD DWG CMDG-D-050
- 42 VEHICLE OPERATOR TO ASSESS MATERIAL ON VEHICLE PRIOR TO EXITING SITE AND

ACID SULFATE SOILS:

- DUE TO THE ELEVATION AND SITE GEOLOGY IT IS UNLIKELY THAT A.S.S. WILL BE ENCOUNTERED ON THIS SITE.
- IF A.S.S. ARE ENCOUNTERED ON THE SITE DURING CONSTRUCTION ENGAGE A SUITABLY 2. QUALIFIED ENVIRONMENTAL CONSULTANT TO PRODUCE AN A.S.S. MANAGEMENT PLAN FOR IT.
- ANY CLEARING OF REMNANT VEGETATION WILL REQUIRE A FAUNA SPOTTER / CATCHER TO BE IN ATTENDANCE.

VEGETATION MANAGEMENT:

WHERE VEGETATION COVENANT EXISTS ON SITE. THIS AREA TO BE CLEARLY PEGGED AND 1 FLAGGED OR FENCED PRIOR TO WORK COMMENCING ON SITE TO PREVENT ANY CLEARING IN THIS AREA.

BUSH FIRE MANAGEMENT:

- THE SITE IS PREDOMINANTLY CLEARED AND NOT IN A BUSH FIRE HAZARD ZONE (BUT STILL MAY BE SUBJECT TO BUSH FIRES)
- ANY CLEARED VEGETATION TO BE MULCHED AND USED ON SITE.
- 3. MULCHED STOCK PILES TO BE NO MORE THAN 2.0m HIGH AND WET DOWN DAILY.
- REMOVE MULCH FROM SITE IF SAFE TO DO SO SHOULD BUSHFIRES THREATEN THE AREA. 4

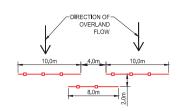
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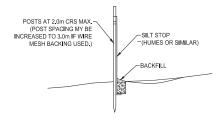
These plans are approved subject to the current conditions of approval associated with Development Permit No.: D/103-2024

Dated: 10 December 2024

DATUM: HORIZ. GDA 94 VERT. AHD		REV	REVISION DESCRIPTION	DATE		DRAFTED	CWR		DWG No.
0 125 25 375 5		A	FOR DISCUSSION FOR APPROVAL	03/05/2024 12/06/2024	ACN 121 309 171 47 Normarby Street	DESIGNED	SJG	NOVUS LOGISTICS	D24.054-C14
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VERTICAL	FOR CONSTRUCTION ONLY WITH COUNCIL APPROVAL				Finite of 49383660 CIVIL /STRUCTURAL DESIGN & PROJECT MANAGEMENT Email: admin/g8dieigh.com.au	RPEQ 7682	SIGN	OPERATIONAL WORKS	REVISION
SCALES m. FULL A3					CIVIL/STRUCTURAL DESIGN & PROJECT MANAGEMENT Email adminigolegn.com.au	11.09.2024	4-X	EROSION CONTROL NOTES	С



SEDIMENT FENCE LAYOUT



SEDIMENT FENCE DETAIL

- FAUNA MANAGEMENT:
- 1.

2024



Adam Doherty

ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

These plans are approved subject to the current conditions of approval associated with

Development Permit No.: D/103-2024

Dated: 10 December 2024

PROPOSED WAREHOUSE AND OFFICE, 11-13 HEMPENSTALL STREET, KAWANA

STORMWATER MANAGEMENT REPORT

FOR NOVUS LOGISTICS

D24.045-RP01



STORMWATER MANAGEMENT PLAN

PROPOSED WAREHOUSE AND OFFICE, 11-13 HEMPENSTALL STREET, KAWANA

Document History & Status

REVISION	DATE	ISSUED TO	DESCRIPTION	BY	APPROVED
A	12/06/2024	Designtek / Novus Logistics	For Comment / Coordination	AD	GB
В	10/09/2024	Rockhampton Regional Council	Information Request response	AD	GB
С	25/09/2024	Rockhampton Regional Council	Further Advice response	AD	GB

Prepared By

Adam Doherty Engineer

Reviewed By

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RPEQ 7682

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Date: Reference: 25/09/2024 D24.054-RP01



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1. Introduction

This report was prepared for Novus Logistics in support of a proposed development to the subject site at 11-13 Hempenstall Street, Kawana QLD 4703. This report should be read in conjunction with the overall application relating to this project. The proponent is seeking approval to develop the lot with a proposed Warehouse and Office operation.

2. Existing Stormwater Conditions

2.1 Internal and Local Government Catchments

2.1.1 Development Site Catchment

The subject area is currently undeveloped and consists of light grass cover with patches of bare earth. The site is practically flat, with average falls of approximately 0.5 to 1% toward the Western boundary to a drainage and services easement where flows are captured in an overland channel which is formed by a concrete barrier kerb and channel. The easement drains to the North towards Frenchman's Creek.

Based on the average flow path slope and assumed fraction impervious of the site, an overall time of concentration (Tc) of 19 minutes has been adopted.

	Friends Equation (Eq 4.5) - Shallow overland sheet flow										
L	Surface	n		tc							
m	Surface	Mannings	%	minutes							
142	Poorly Grassed	0.035	1.1	19							

Table 1

The existing area has a fraction impervious of 0 (zero) in accordance with QUDM Table 4.5.1 and a corresponding C10 value of 0.70 in accordance with QUDM Table 4.5.4 - C10 values for zero fraction impervious.

Utilising a Tc of 19 minutes and the relevant rainfall intensities, the following discharges for a range of events were calculated using the C10 value of 0.70 where Qy=C*I*A/360 for the existing site.

Dev	elopment Area	0.3039	ha		Fi	0.000
AEP	С	I.	Α	Q	¹ I ₁₀ (mm/hr)	65.1
%	coefficient	mm/hr	ha	m³/s	TC (minutes)	19
63.2	0.560	73.6	0.304	0.035	C ₁₀	0.700
50	0.595	81.8	0.304	0.041	From QUDM Table 4.5.4	
20	0.665	108.0	0.304	0.061		
10	0.700	127.0	0.304	0.075		
5	0.735	146.0	0.304	0.091		
2	0.805	173.0	0.304	0.118]	
1	0.840	193.0	0.304	0.137	In accordance with QUDM E	qn. 4.3

Table 2

Refer drawings in Appendix A for Stormwater Management Strategy Drawings.



2.2 External Catchments

2.2.1 Existing External Catchment

The existing drainage easement commands flows from two properties to the South, being Lot 57 and Lot 2. It is assumed that the existing easement is appropriately sized for the upstream flows, with the easement being maintained and the subject site not releasing any additional flows to the easement post development the easement will continue to convey the upstream catchment flows as it does in the pre-developed scenario.



Table 1 – Extent of existing upstream catchment

3. Post Developed Site Flows and Management

3.1 Post Developed Flows

The proposed development of the site increases the fraction impervious to a value of 0.796 based on information provided by the applicant. Using the post developed fraction impervious, a C_{10} value of 0.849 (From QUDM Table 4.5.3) was adopted.

As this is a commercial site with a reasonably high Impervious area a single time of concentration of 5 minutes was adopted for all elements of the post development calculations.

Based on the revised fraction impervious and revised time of concentration the following discharges from site were calculated:



POST-DE	POST-DEVELOPMENT SITE CONDITIONS											
Develop	Development Area		ha		Fi 0.796							
AEP	С	I	Α	Q	¹ I ₁₀ (mm/hr) 65.1							
%	coefficient	mm/hr	ha	m³/s	TC (minutes) 5							
63.2	0.679	115.0	0.3039	0.0659	C ₁₀ 0.849							
50	0.722	128.0	0.3039	0.0780	From QUDM Table 4.5.3							
20	0.806	170.0	0.3039	0.1157								
10	0.849	200.0	0.3039	0.1433								
5	0.891	229.0	0.3039	0.1723								
2	0.976	268.0	0.3039	0.2209								
1	1.000	300.0	0.3039	0.2533	In accordance with QUDM Eqn. 4.3							

Table 5

When compared with the pre-developed total site flows, we note an increase in flow for all recurrence intervals. Refer table below:

COMPARISON OF UNTREATED FLOWS			
Event	Pre-	Post-	
AEP	Development	Development	Change
%	m³/s	m³/s	%
63.2	0.0348	0.0659	89%
50	0.0411	0.0780	90%
20	0.0606	0.1157	91%
10	0.0750	0.1433	91%
5	0.0906	0.1723	90%
2	0.1176	0.2209	88%
1	0.1369	0.2533	85%

Table 6

3.2 Discharge Flow Management

3.2.1 Quantity Mitigation

It is proposed to mitigate the increase in site runoff by providing on-site detention (OSD) capturing all post developed internal site flows from impervious areas.

Two OSD devices are proposed, 1 x 10kL rainwater tank (or two 5kL tanks in series) capturing roof water from Shed 1 & 2 which will discharge through an orifice outlet of 70mm diameter to an internal underground stormwater line, routed to the detention basin. The larger detention basin that will be constructed in the lower Northwestern corner of the development area, adjacent to shed 4 and will receive all flows from Shed 3 & 4 and all impervious areas in addition to the detention tank outflows.

Both OSD have been analysed for a range of events from 50% AEP through to a 1% AEP using Autodesk Hydra flow Hydrographs. The maximum required storage volume of 43.7 kL is required for the detention basin and 10kL for the detention tank.

With the above detention in place a total site discharge reduction was achieved across all events.



	Post Developed Treated Flows							
Event AEP	Pre- Development	Post- Development	- routed tank inflow	+ routed tank outflow to basin	- routed basin in. (site + tank outflow)	+ Routed basin out	Total Outflow	Rear Of Site Post Dev Discharge
%	m3/s	m3/s	m3/s	m3/s	m3/s	m3/s	m3/s	m3/s
50	0.041	0.0780	0.0109	0.004	0.059	0.026	0.0341	-27%
20	0.061	0.1157	0.0162	0.005	0.0864	0.035	0.0482	-29%
10	0.075	0.1433	0.0200	0.005	0.105	0.042	0.0603	-26%
5	0.091	0.1723	0.0240	0.006	0.127	0.056	0.0773	-21%
2	0.118	0.2209	0.0298	0.006	0.156	0.079	0.1141	-8%
1	0.137	0.2533	0.0333	0.006	0.1730	0.093	0.1399	-2%

Table 7

Refer to Appendix B for routing Hydrographs.

3.3 Stormwater Quality Management

Due to the size of the development (>2500m²), State Planning Policy Healthy Water is triggered.

A water quality model was developed using the MUSIC stormwater quality software. The treatment train consists of the following measures.

- Ecosol Net Guard (GPT) on the inlet to the detention basin.
- Enviro Australis E30 unit to roofwater line
- Roof water and impervious area detention basin(s).
- Rock Mulch (pervious) treatment to the Southern perimeter of the site.

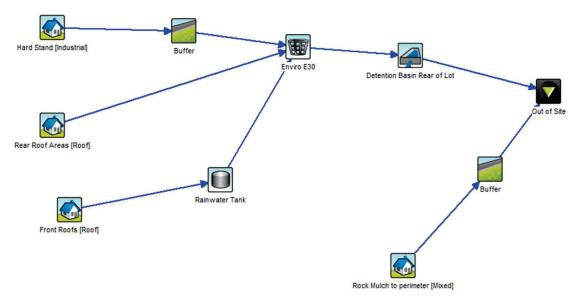


Figure 1 – Treatment Train schematic



compared to SFF targets for Central Queensiand South.				
COMPARISON OF TREATMENT TRAIN EFFECTIVENESS				
POLUTANT SPP TARGET ACHIEVED				
FOLOTAINT	(%)	REDUCTION (%)		
Suspended Solids	85	83.1		
Total Phosphorous	60.0	49.1		
Total Nitrogen	45.0	55.1		
Gross Pollutants	90.0	100.0		

The treatment train effectiveness did not meet the requirements of the State Planning Policy for Central Queensland South, refer to table 8 below for treatment train effectiveness compared to SPP targets for Central Queensland South.

Table 8

The above table notes that although targets for SS and TP are not achieved, the theoretical reductions are close to target. No other practical opportunities were available on the site in this instance given that the site does not have any ability to discharge below the existing ground level at the drainage easement at the rear of the site. It is believed that all reasonable effort has been made to treat stormwater runoff from the site and that post-construction testing will yield pollutant loadings typical to the surrounding development area.

4. Conclusion

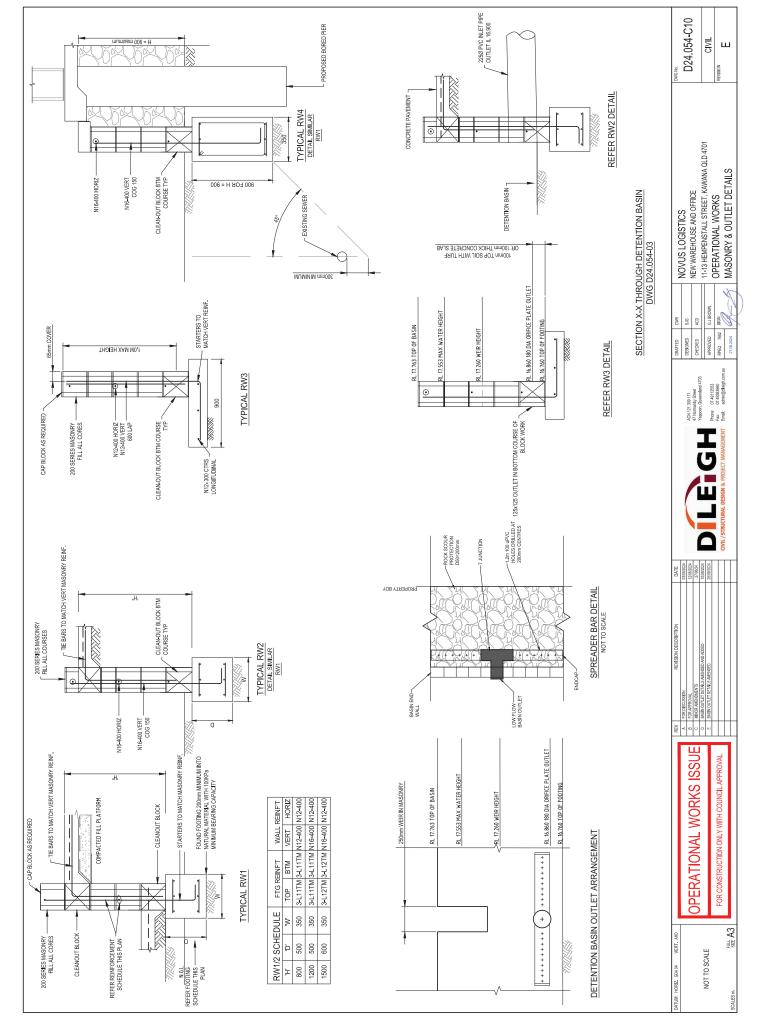
As the proposed development will increase the impervious area of the site it is proposed to mitigate the increase in runoff by providing a detention/retention basin to mitigate the increase in peak flows from the proposed development. Water Quality will also be managed through a series of quality improvement measures however will not meet SPP objectives in this instance.

With the development not discharging any additional flows post development due to the proposed measures reduction strategy and the State Planning Policy quality targets not being met even with implementation of SQIDs, we request a relaxation as it was not practical to achieve all targets in this instance due to a lack of existing discharge opportunities to the existing drainage easement.

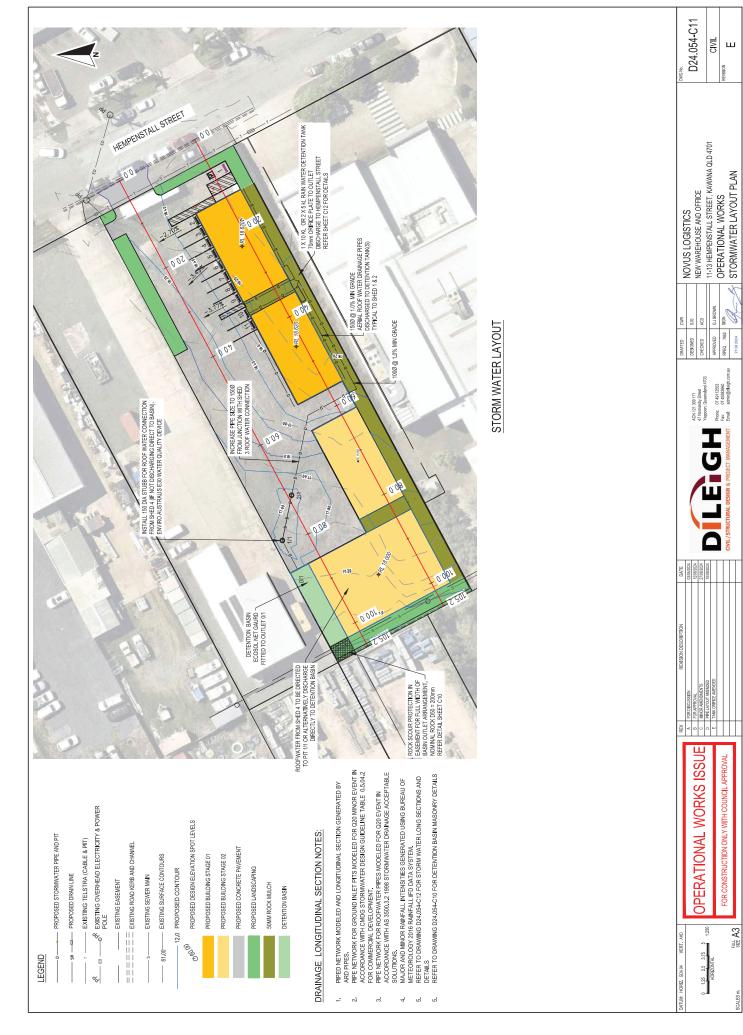
Adam Doherty For and On Behalf of Dileigh Consulting Engineers Pty Ltd

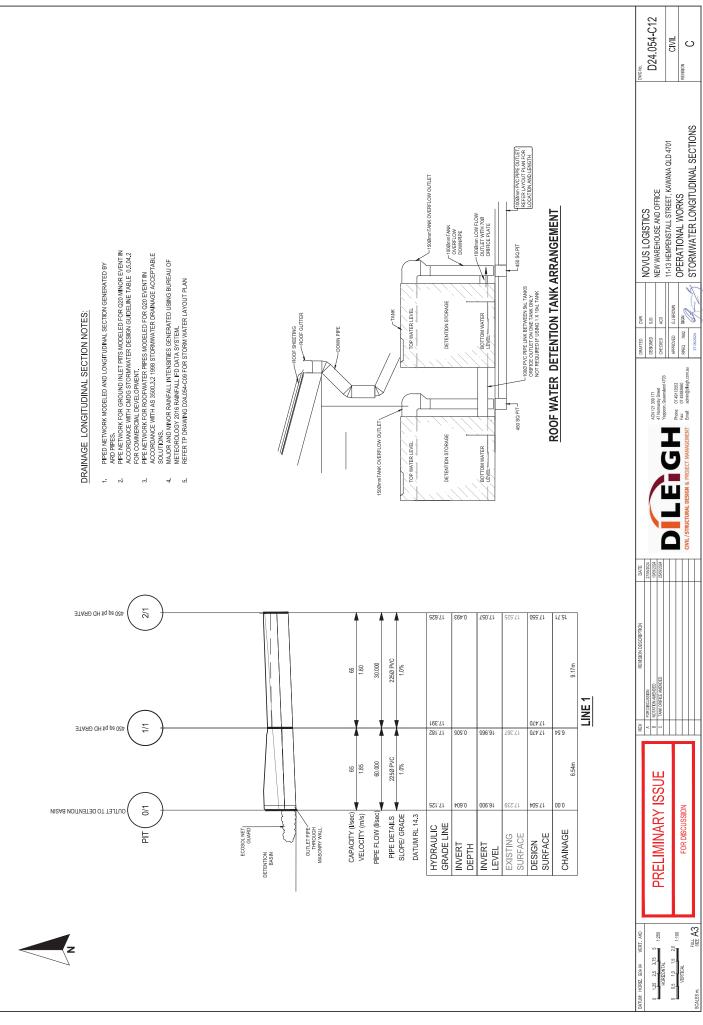


Appendix A – Stormwater Management Strategy Drawings



C:)OnoDrive - DILEICH CONSULTING ENGINEERS FTY LTDIDIBAI2024/D24-054_NOVUS - 11-13 Homponshill 51, Kawana)C Draw/CriviD54.054, OPW dwg, 10, 550935054 3-4525 PW, Patholpt, 150 full bioed P3 (420.00 × 257, 100 MW); 1:1





2024



Adam Doherty

ROCKHAMPTON REGIONAL COUNCIL

APPROVED PLANS

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Development Permit No.: D/103-2024

Dated: 10 December 2024

PROPOSED WAREHOUSE AND OFFICE, 11-13 HEMPENSTALL STREET, KAWANA

STORMWATER MANAGEMENT REPORT

FOR NOVUS LOGISTICS

D24.045-RP01

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

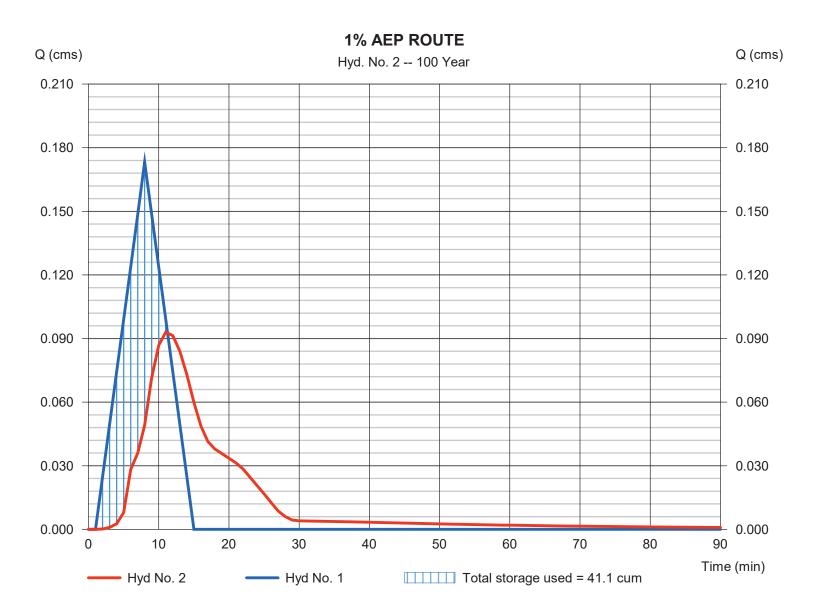
Wednesday, 09 / 25 / 2024

Hyd. No. 2

1% AEP ROUTE

Hydrograph type	= Reservoir	Peak discharge	= 0.093 cms
Storm frequency	= 100 yrs	Time to peak	= 11 min
Time interval	= 1 min	Hyd. volume	= 72.7 cum
Inflow hyd. No.	= 1 - 1% AEP	Max. Elevation	= 17.51 m
Reservoir name	= DET1	Max. Storage	= 41.1 cum

Storage Indication method used.



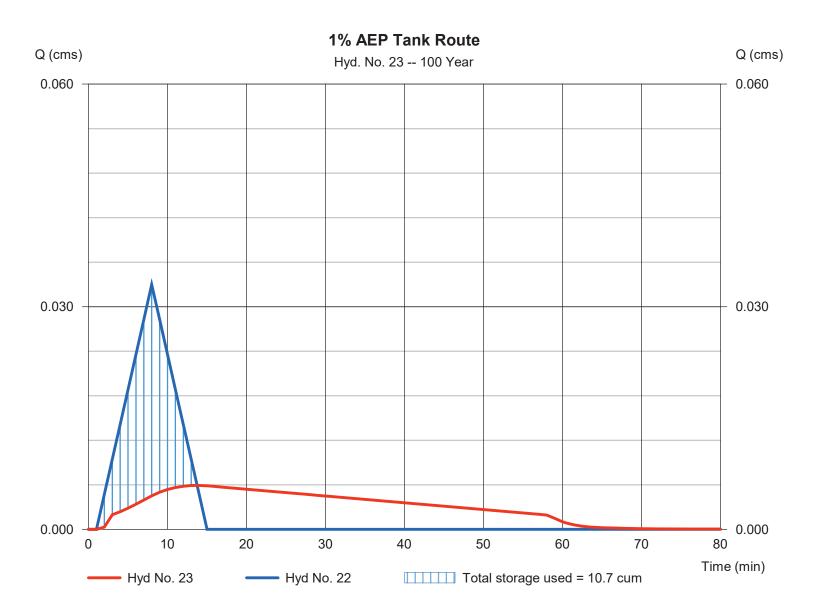
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

Hyd. No. 23

1% AEP Tank Route

Hydrograph type	= Reservoir	Peak discharge	= 0.006 cms
Storm frequency	= 100 yrs	Time to peak	= 14 min
Time interval	= 1 min	Hyd. volume	= 13.8 cum
Inflow hyd. No.	= 22 - Roof to tank 1% AEP	Max. Elevation	= 18.65 m
Reservoir name	= 10kL Tank	Max. Storage	= 10.7 cum

Storage Indication method used.



Wednesday, 09 / 25 / 2024

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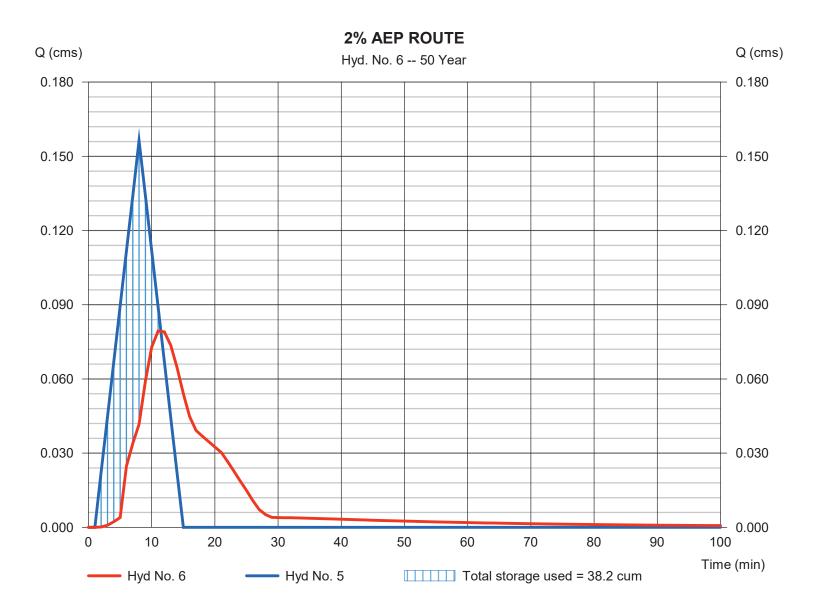
Wednesday, 09 / 25 / 2024

Hyd. No. 6

2% AEP ROUTE

Hydrograph type	= Reservoir	Peak discharge	= 0.079 cms
Storm frequency	= 50 yrs	Time to peak	= 11 min
Time interval	= 1 min	Hyd. volume	= 65.6 cum
Inflow hyd. No.	= 5 - 2% AEP	Max. Elevation	= 17.47 m
Reservoir name	= DET1	Max. Storage	= 38.2 cum

Storage Indication method used.



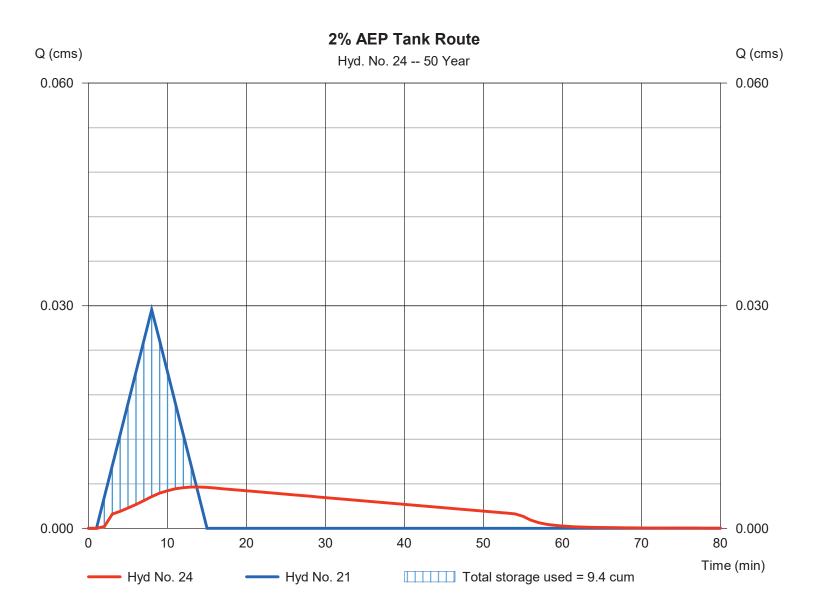
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Hyd. No. 24

2% AEP Tank Route

Hydrograph type	= Reservoir	Peak discharge	= 0.006 cms
Storm frequency	= 50 yrs	Time to peak	= 14 min
Time interval	= 1 min	Hyd. volume	= 12.4 cum
Inflow hyd. No.	= 21 - Roof to tank 2% AEP	Max. Elevation	= 18.43 m
Reservoir name	= 10kL Tank	Max. Storage	= 9.4 cum

Storage Indication method used.



Wednesday, 09 / 25 / 2024

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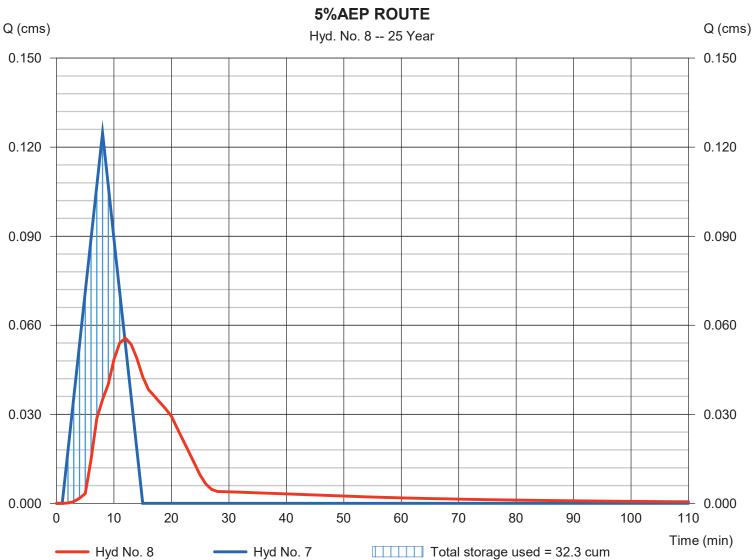
Wednesday, 09 / 25 / 2024

Hyd. No. 8

5%AEP ROUTE

Hydrograph type	= Reservoir	Peak discharge	= 0.056 cms
Storm frequency	= 25 yrs	Time to peak	= 12 min
Time interval	= 1 min	Hyd. volume	= 52.4 cum
Inflow hyd. No.	= 7 - 5% AEP	Max. Elevation	= 17.37 m
Reservoir name	= DET1	Max. Storage	= 32.3 cum

Storage Indication method used.



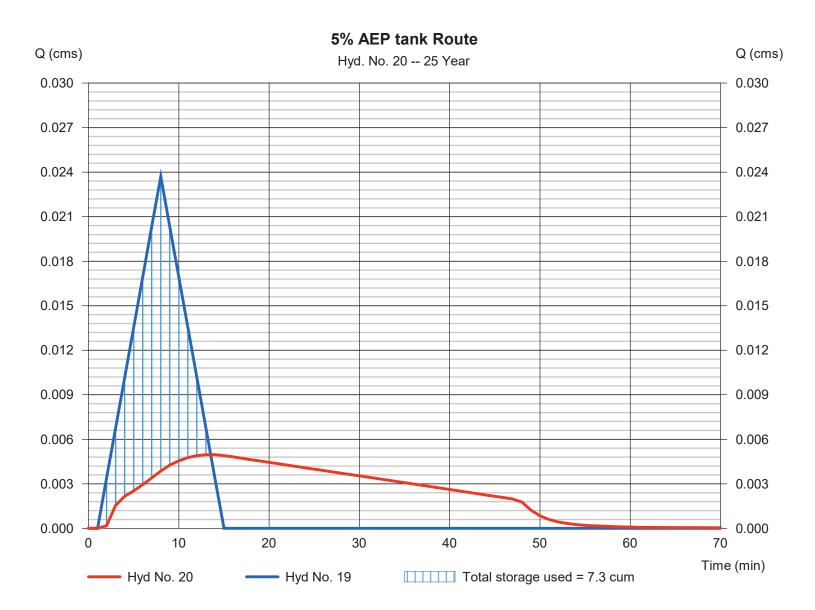
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Hyd. No. 20

5% AEP tank Route

Reservoir	Peak discharge	= 0.005 cms
25 yrs	Time to peak	= 14 min
1 min	Hyd. volume	= 9.9 cum
19 - Roof to tank 5% AEP	Max. Elevation	= 18.08 m
10kL Tank	Max. Storage	= 7.3 cum
	25 yrs 1 min 19 - Roof to tank 5% AEP	25 yrsTime to peak1 minHyd. volume19 - Roof to tank 5% AEPMax. Elevation

Storage Indication method used.



Wednesday, 09 / 25 / 2024

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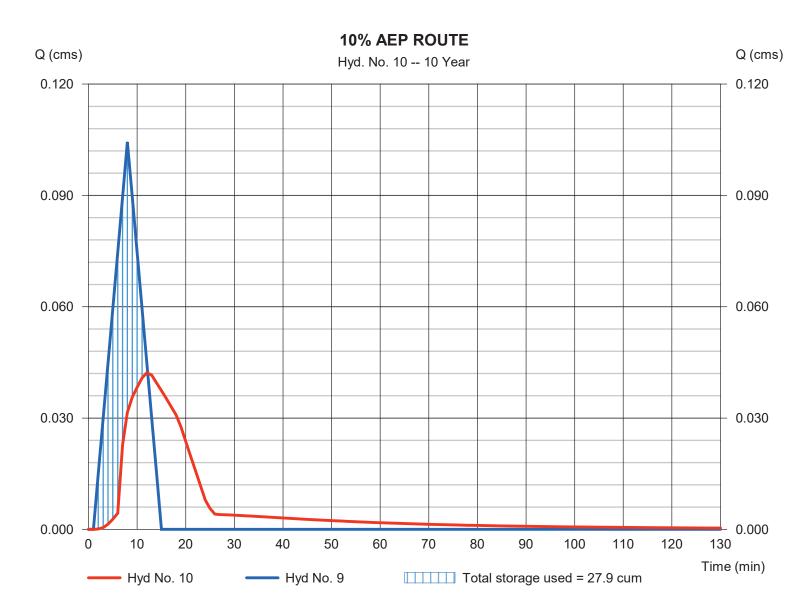
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Hyd. No. 10

10% AEP ROUTE

42 cms
min
.8 cum
.30 m
9 cum

Storage Indication method used.



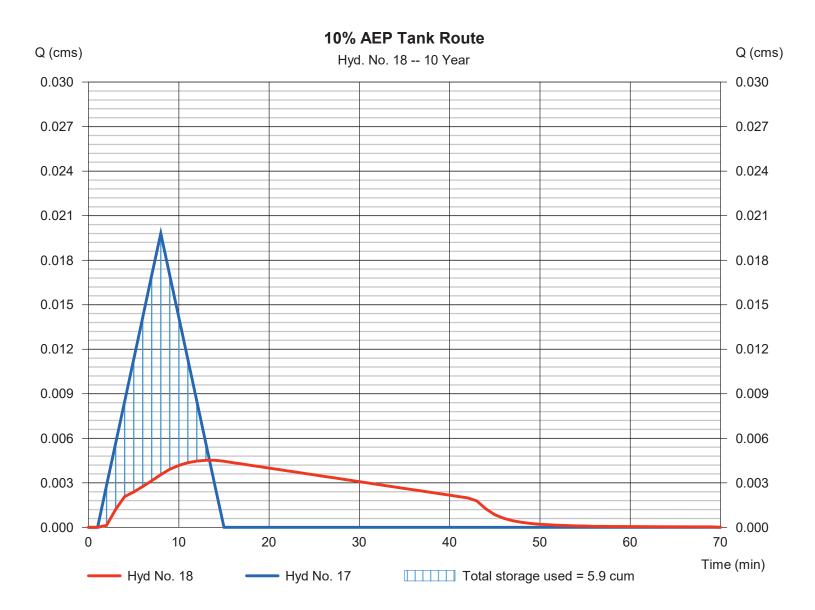
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

Hyd. No. 18

10% AEP Tank Route

Hydrograph type	= Reservoir	Peak discharge	= 0.005 cms
Storm frequency	= 10 yrs	Time to peak	= 13 min
Time interval	= 1 min	Hyd. volume	= 8.3 cum
Inflow hyd. No.	= 17 - Roof to tank 10% AEP	Max. Elevation	= 17.85 m
Reservoir name	= 10kL Tank	Max. Storage	= 5.9 cum

Storage Indication method used.



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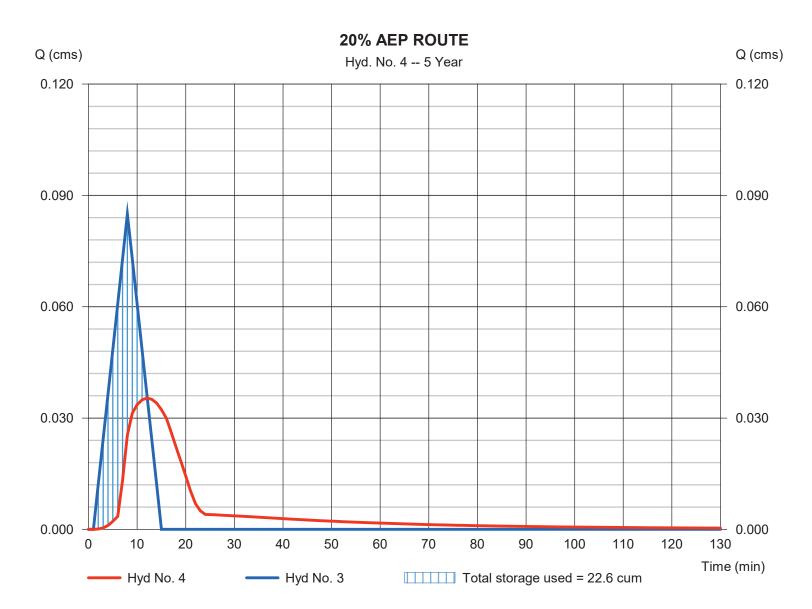
Wednesday, 09 / 25 / 2024

Hyd. No. 4

20% AEP ROUTE

Hydrograph type	= Reservoir	Peak discharge	= 0.035 cms
Storm frequency	= 5 yrs	Time to peak	= 12 min
Time interval	= 1 min	Hyd. volume	= 35.6 cum
Inflow hyd. No.	= 3 - 20% AEP	Max. Elevation	= 17.22 m
Reservoir name	= DET1	Max. Storage	= 22.6 cum

Storage Indication method used.



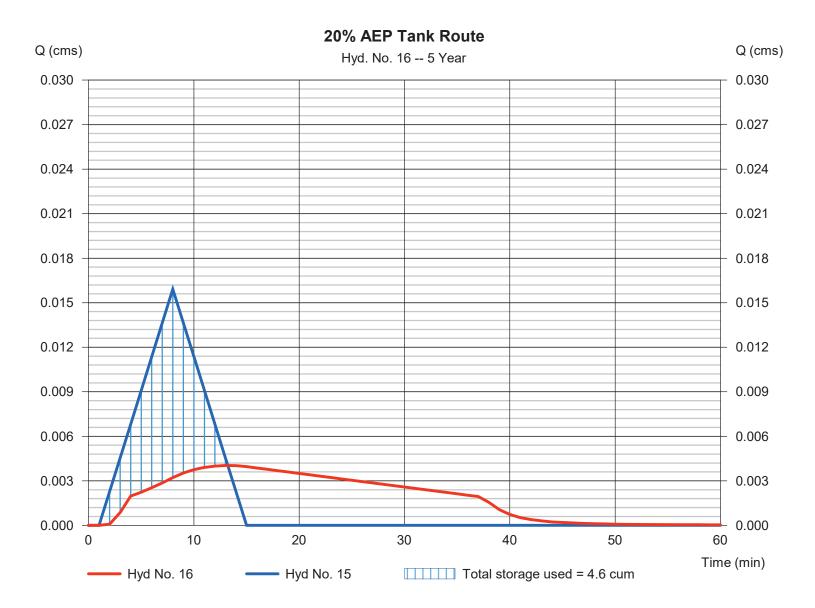
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

Hyd. No. 16

20% AEP Tank Route

Hydrograph type	= Reservoir	Peak discharge	= 0.004 cms
Storm frequency	= 5 yrs	Time to peak	= 13 min
Time interval	= 1 min	Hyd. volume	= 6.7 cum
Inflow hyd. No.	= 15 - Roof to tank 20% AEP	Max. Elevation	= 17.62 m
Reservoir name	= 10kL Tank	Max. Storage	= 4.6 cum

Storage Indication method used.



Wednesday, 09 / 25 / 2024

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

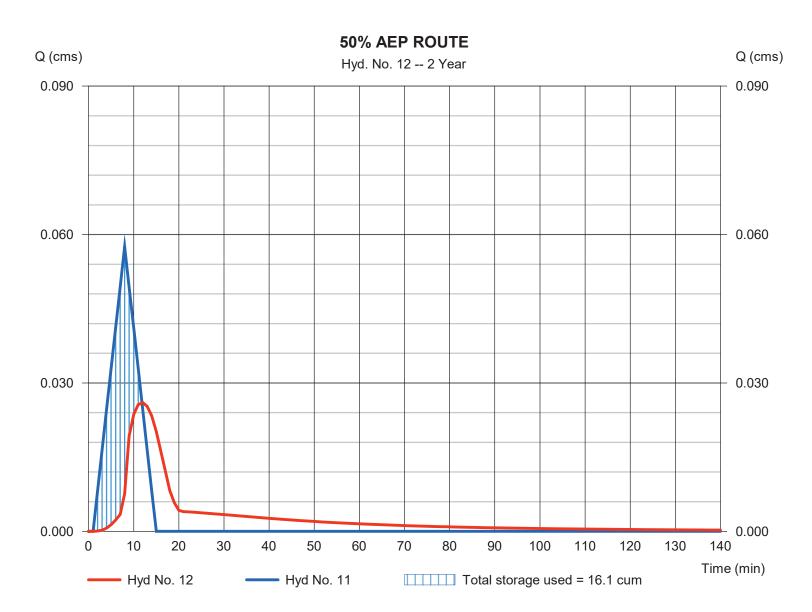
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Hyd. No. 12

50% AEP ROUTE

Hydrograph type	= Reservoir	Peak discharge	= 0.026 cms
Storm frequency	= 2 yrs	Time to peak	= 12 min
Time interval	= 1 min	Hyd. volume	= 24.1 cum
Inflow hyd. No.	= 11 - 50% AEP	Max. Elevation	= 17.12 m
Reservoir name	= DET1	Max. Storage	= 16.1 cum

Storage Indication method used.



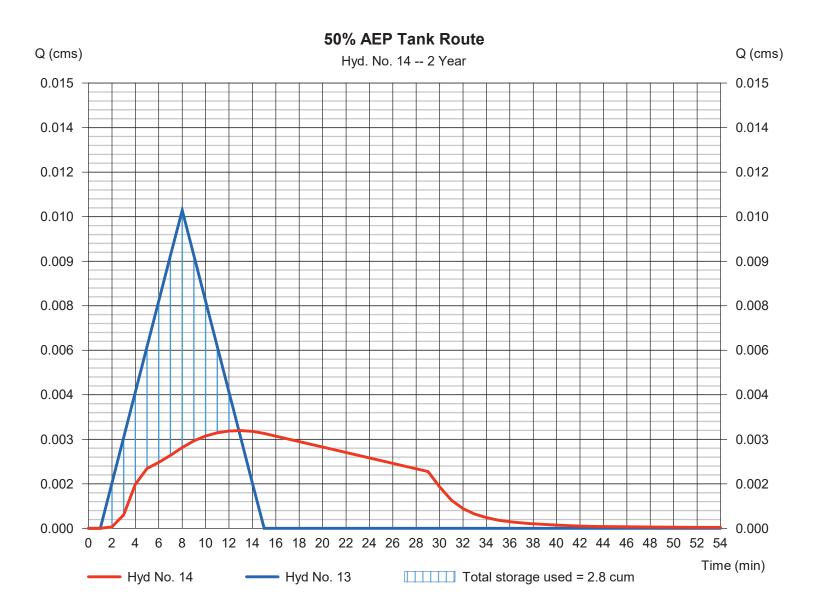
Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2024

Hyd. No. 14

50% AEP Tank Route

Hydrograph type	= Reservoir	Peak discharge	= 0.003 cms
Storm frequency	= 2 yrs	Time to peak	= 13 min
Time interval	= 1 min	Hyd. volume	= 4.5 cum
Inflow hyd. No.	= 13 - Roof to tank 50% AEP	Max. Elevation	= 17.32 m
Reservoir name	= 10kL Tank	Max. Storage	= 2.8 cum

Storage Indication method used.



Wednesday, 09 / 25 / 2024